

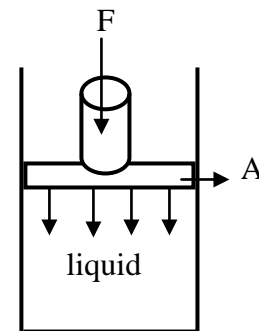
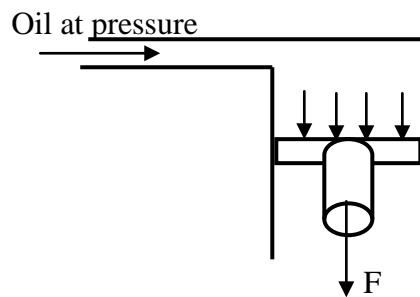
# Fluid Statics

- Fluid Statics deals with problems associated with fluids at rest.
- In fluid statics, there is no relative motion between adjacent fluid layers.
- Therefore, there is no shear stress in the fluid trying to deform it.
- The only stress in fluid statics is *normal stress*
  - ✓ Normal stress is due to pressure
  - ✓ Variation of pressure is due only to the weight of the fluid → fluid statics is only relevant in presence of gravity fields.
- Applications: Floating or submerged bodies, water dams and gates, liquid storage tanks, etc.

• Pressure : is the Normal force per unit area

$$p = \frac{F}{A}$$

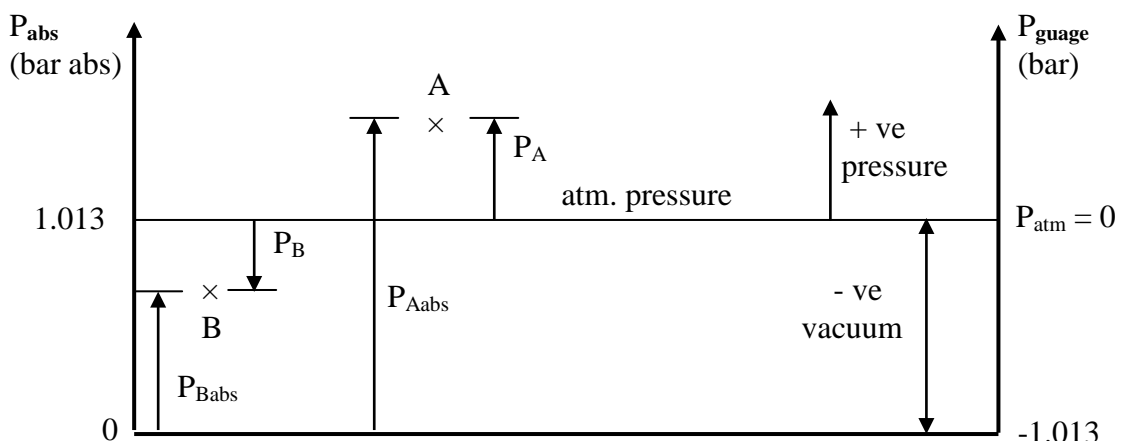
$$Pa = \frac{N}{m^2}$$



## \* Absolute, atmospheric and guage pressure

### Absolute pressure

$P_{abs}$   
(bar abs)



### guage pressure

$P_{guage}$   
(bar)

Absolute pressure = true pressure

$$P_{abs} = P_{guage} + P_{atm}$$

\* All given values for pressure are gauge except if :

1. (abs) is mentioned beside the unit.
2. Dealing with atmospheric pressure.
3. Dealing with vapour pressure.

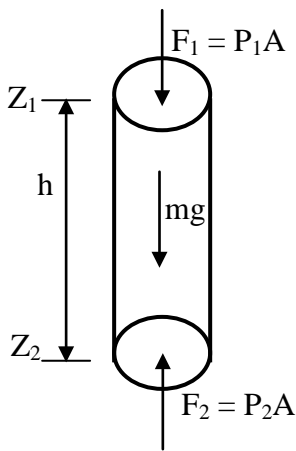
\* No pressure gauge value less than -1.013 bar

\* 1 bar =  $10^5$  pascal

\* -ve pressure is called vacuum

**\* In a static liquid :**

1. The pressure, at a certain point, is the same in all directions.
2. The pressure is constant in the same horizontal plane.
3. The pressure changes in the vertical direction.



$$\rho = \frac{m}{V} \quad \therefore m = \rho V = \rho Ah$$

$$\sum F_y = 0 \quad \downarrow +$$

$$F_1 + mg - F_2 = 0$$

$$P_1 A + \rho A(Z_2 - Z_1)g - P_2 A = 0 \quad \div A$$

$$P_1 + \rho hg - P_2 = 0$$

$$\boxed{P_2 - P_1 = \rho g h} \quad \text{or} \quad \boxed{P_2 - P_1 = \gamma h}$$

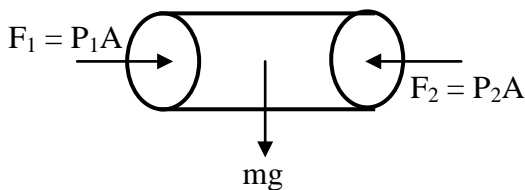
$$\rightarrow + \sum F_x = 0$$

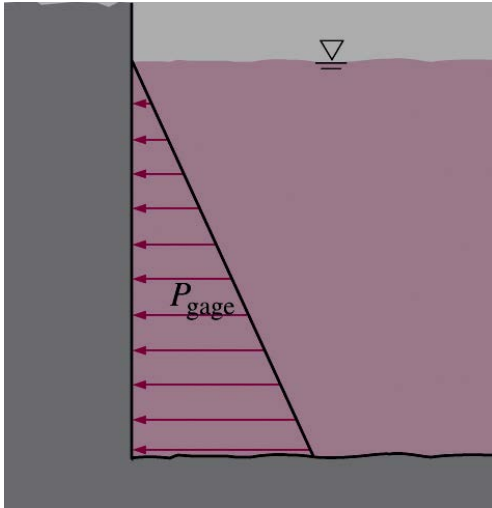
$$F_1 - F_2 = 0$$

$$P_1 A - P_2 A = 0 \quad \div A$$

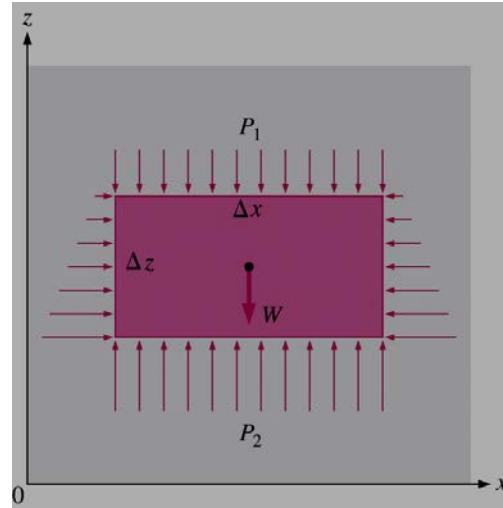
$$P_1 - P_2 = 0$$

$$\boxed{P_1 = P_2}$$



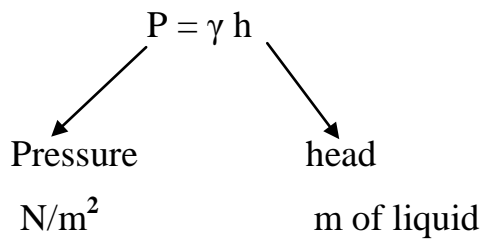


The pressure of a fluid at rest increases with depth (as a result of added weight).



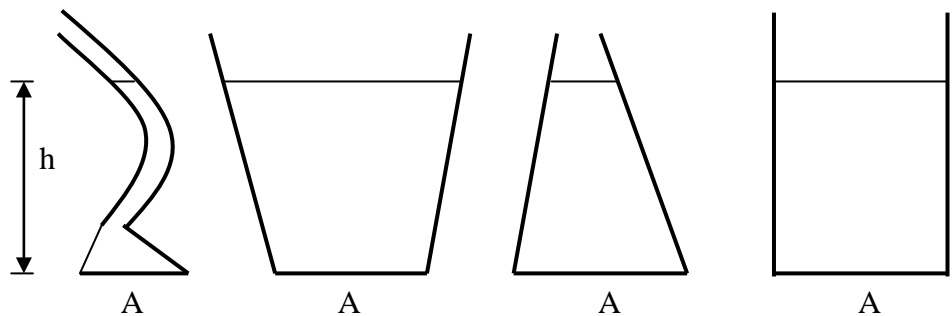
Free-body diagram of a rectangular fluid element in equilibrium.

**\* Pressure and head**



\* head : is the vertical length that can define the pressure.

**\*The hydrostatic paradox :**



$$P_{\text{bottom}} = \gamma h \quad \text{then} \quad F_{\text{bottom}} = PA = \gamma hA$$

Although the weight of fluid is different, the force in the base of the four vessels is the same. This force depends on the depth (h) and the base area A.

### Example :

A cylinder contains a fluid at pressure of  $350 \text{ KN/m}^2$

- Express the pressure in terms of a head of :-

a) Water  $\rho_w = 1000 \text{ kg/m}^3$       b) Mercury  $\text{SG}_m = 13.6$

- Determine the absolute pressure if  $P_{\text{atm}} = 101.3 \text{ KN/m}^2$  ?

### Solution

$$P = \gamma h = \rho g h$$

a)  $350 * 10^3 = 1000 * 9.81 * h_w \quad \therefore h_w = 35.68 \text{ m of water}$

b)  $P = \text{SG}_m \rho_w g h \quad \text{SG}_m = \frac{\rho_m}{\rho_w} \quad \rho_m = \text{SG}_m \rho_w$

$350 * 10^3 = 13.6 * 1000 * 9.81 * h_m \quad \therefore h_m = 2.62 \text{ m of mercury}$

$$\begin{aligned} P_{\text{abs}} &= P_{\text{gage}} + P_{\text{atm}} \\ &= 350 * 10^3 + 101.3 * 10^3 \\ &= 451300 \text{ N/m}^2 * 10^{-3} \\ &= 451.3 \text{ KN/m}^2 \end{aligned}$$

### Example :

If  $h_{\text{atm}} = 76 \text{ cm Hg}$ , determine  $P_{\text{atm}}$ ?

### Solution

$$\begin{aligned} P_{\text{atm}} &= \gamma_m h & \text{SG}_m &= \frac{\gamma_m}{\gamma_w} \\ &= \text{SG}_m \gamma_w h & \gamma_m &= \text{SG}_m \gamma_w \\ &= 13.6 * 9800 * (76 * 10^{-2}) \\ &= 1.013 * 10^5 \text{ N/m}^2 \end{aligned}$$

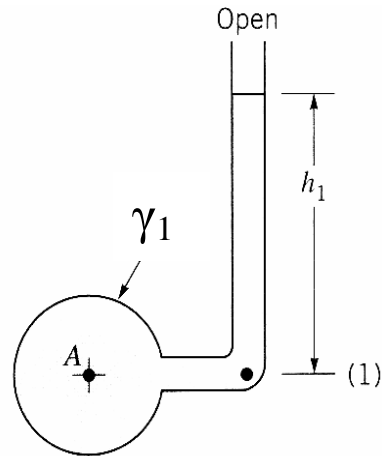
**\* pressure measurements by manometers**

**\* Piezometer**

Pressure tube or piezometer

Consists of a single vertical tube

$$P_A = \gamma_1 h_1$$



**\* U- tube manometer**

Statics

Same horizontal plane

\* to make pressure equivalence

1 – Still liquid

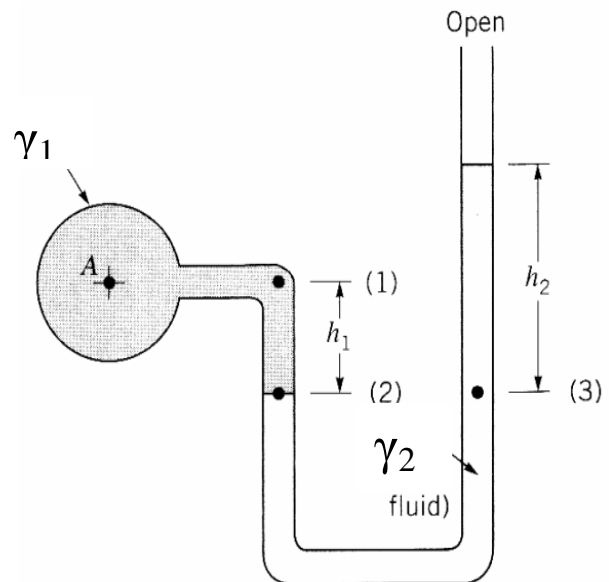
2 – Continues liquid

3 – Same liquid

$$P_I = P_{II}$$

$$P_A + \rho_1 g h_1 = \rho_2 g h_2$$

$$P_A + \gamma_1 h_1 = \gamma_2 h_2$$



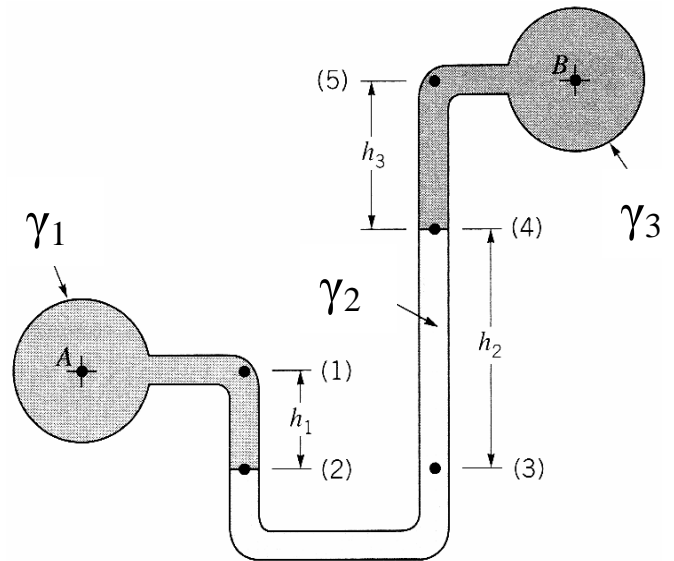
**U-tube manometer**

$$P_I = P_{II}$$

$$P_A + \rho_1 g h_1 = P_B + \rho_2 g h_2 + \rho_3 g h_3$$

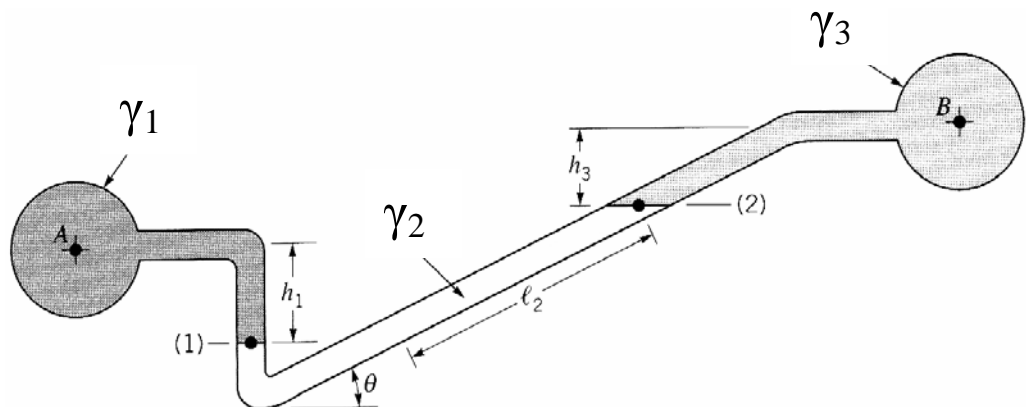
$$P_A - P_B = \rho_2 g h_2 + \rho_3 g h_3 - \rho_1 g h_1$$

$$P_A - P_B = \gamma_2 h_2 + \gamma_3 h_3 - \gamma_1 h_1$$



**Differential U-tube manometer.**

**\* Inclined-Tube Manometer**



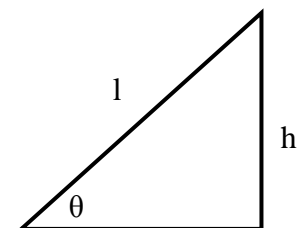
**Inclined-tube Manometer.**

$$P_I = P_{II}$$

$$P_A + \rho_1 g h_1 = P_B + \rho_2 g l_2 \sin \theta + \rho_3 g h_3$$

$$P_A - P_B = \rho_2 g l_2 \sin \theta + \rho_3 g h_3 - \rho_1 g h_1$$

$$P_A - P_B = \gamma_2 l_2 \sin \theta + \gamma_3 h_3 - \gamma_1 h_1$$



$$\sin \theta = \frac{h}{l}$$

$$h = l \sin \theta$$

**\* U-tube with one enlarged**

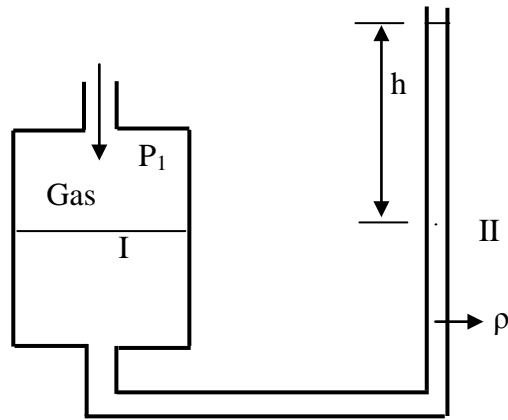
volume = volume

$$A * ll = a * h$$

$$ll = \frac{a}{A} * h$$

$$= \frac{\frac{\pi}{4} d^2}{\frac{\pi}{4} D^2} * h$$

$$ll = \frac{d^2}{D^2} * h$$

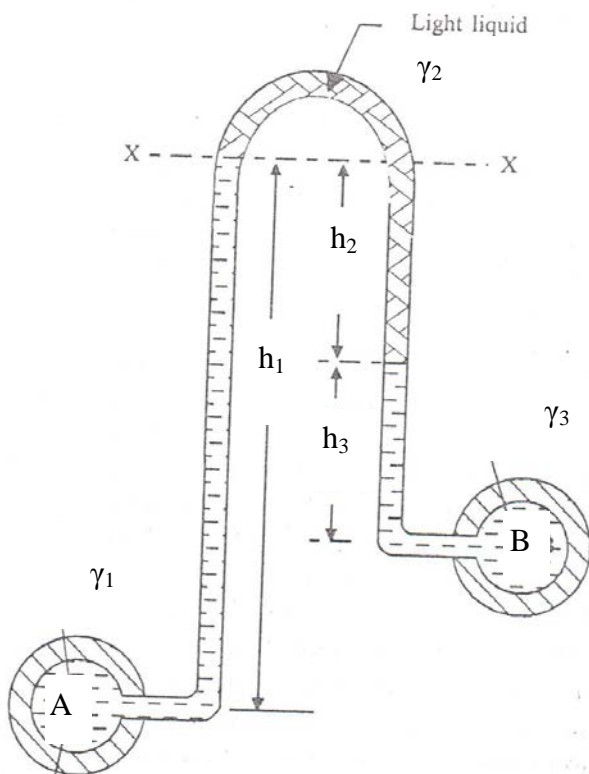


$$P_I = P_{II}$$

$$P_I = \rho g ll + \rho g h$$

$$= \rho g * \frac{d^2}{D^2} h + \rho g h$$

$$= \rho g h \left( \frac{d^2}{D^2} + 1 \right)$$



**\* Inverted U-tube**

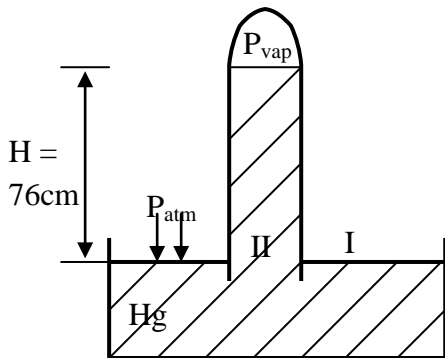
$$P_I = P_{II}$$

$$P_A - \rho_1 g h_1 = P_B - \rho_3 g h_3 - \rho_2 g h_2$$

$$P_A - P_B = \rho_1 g h_1 - \rho_3 g h_3 - \rho_2 g h_2$$

$$\Delta P =$$

**\*Atmospheric pressure (Barometric pressure)**



$$P_{vap_{Hg}} = 1.7 * 10^{-5} \text{ bar}$$

$$= 1.7 \frac{N}{m^2} \approx 0 \text{ neglected}$$

$$P_I = P_{II}$$

$$P_{atm} = P_{vap_{hg}} + \rho_m gH$$

$$= 13600 * 9.8 * 0.76$$

$$= 1.013 * 10^5 \text{ N/m}^2$$

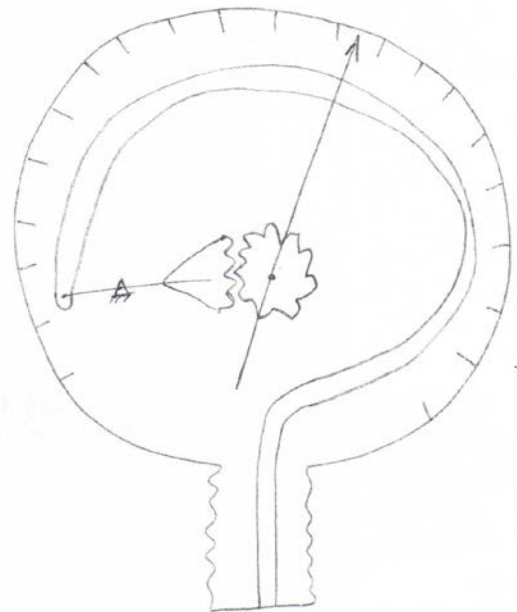
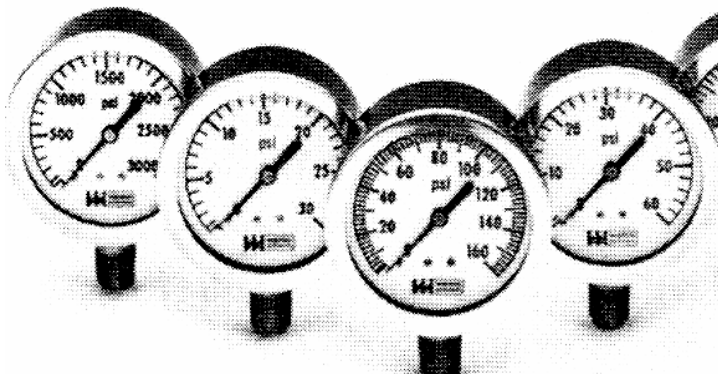
$$= 1.013 \text{ bar}$$

**\* Bourdon tube gauge**

It is used for measuring pressure in almost all ranges except minutely small pressure.

Disadvantages:

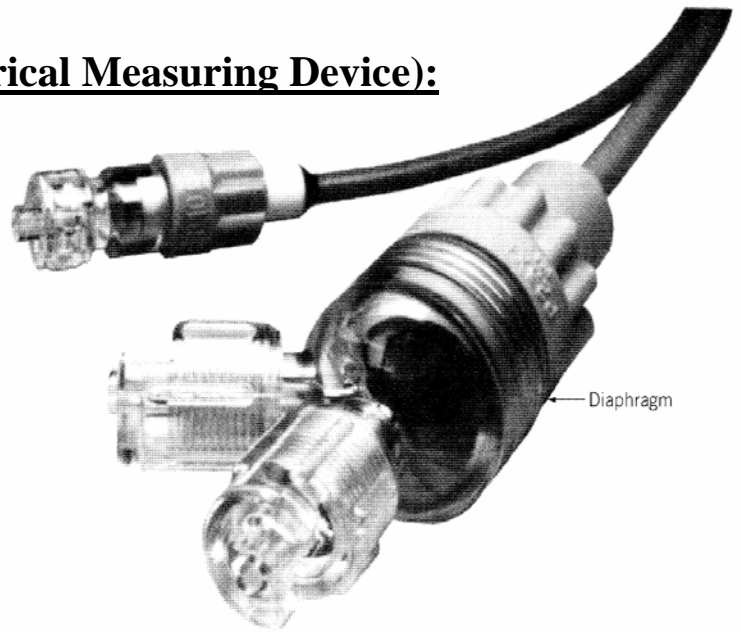
- 1 – Needs calibration on dead weight tester.
- 2 – Accuracy is less than liquid Columns.





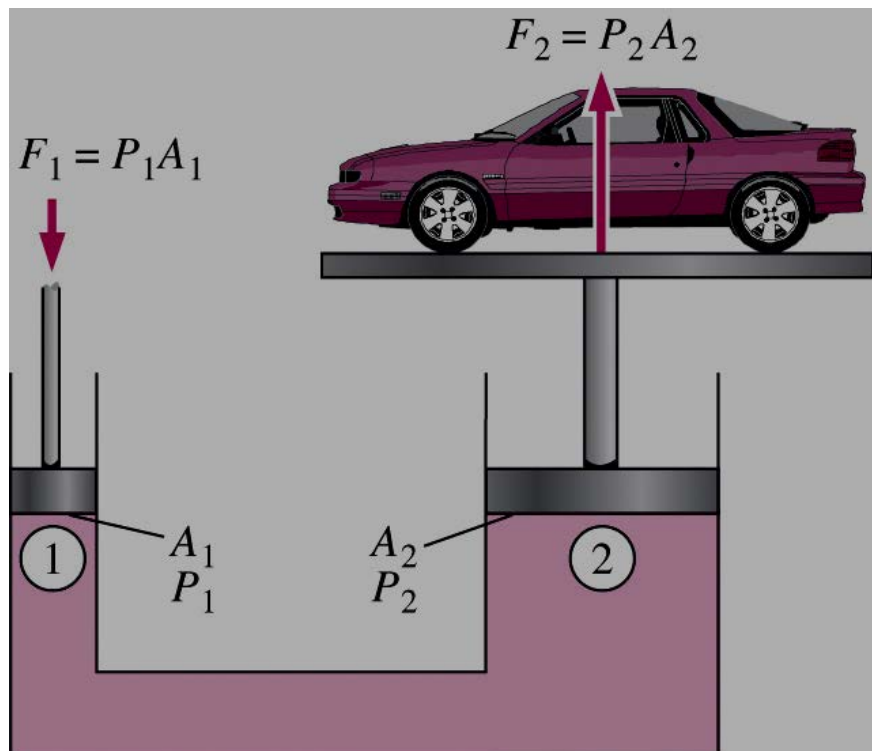
**\* Pressure Transducer (Electrical Measuring Device):**

A pressure transducer converts pressure into an electrical output.



**Applications**

Lifting of a large weight by a small force by the application of Pascal's law.



**Pascal's law:** The pressure applied to a confined fluid increases the pressure throughout by the same amount.

$$P_1 = P_2 \quad \rightarrow \quad \frac{F_1}{A_1} = \frac{F_2}{A_2} \quad \rightarrow \quad \frac{F_2}{F_1} = \frac{A_2}{A_1}$$

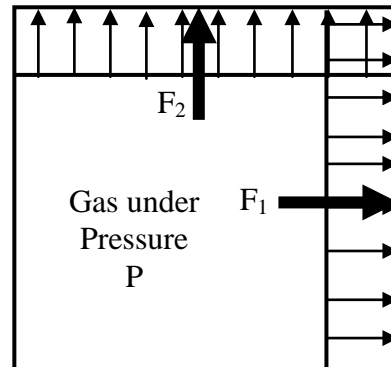
# Hydrostatic Forces on Plane Surfaces

## \* Forces due to fluid pressure on Flat surface

### \* For gases

$F_1$  &  $F_2$  perpendicular on the surface & acts at the center of area subjected to pressure

$$F_1 = PA_1 \quad \& \quad F_2 = PA_2$$



### \* for liquids

\*  $F_1$  = volume of pressure prism

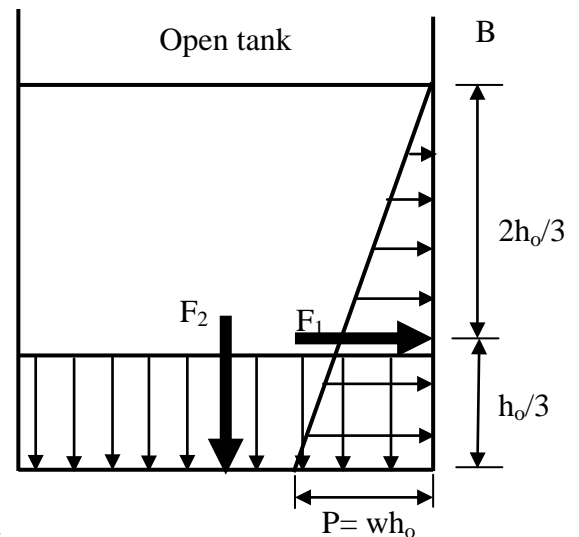
$$= \gamma h_0 * \frac{h_0}{2} * B$$

$$= \frac{\gamma h_0^2 B}{2}$$

\*  $F_1$  acts at the center of volume of the prisme  $\perp$  to the surface

\*  $F_2$  (on bottom) =  $PA = \gamma h_0 * A$

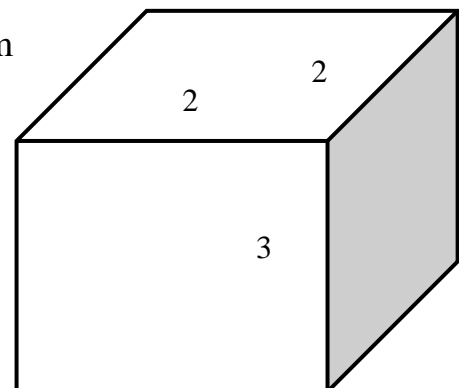
\*  $F_2$  acts  $\perp$  on bottom and at center of area



### Example :

A square tank  $(2 \times 2) \times 3$  m high. Calculate the force on one of the vertical sides of the tank and in its bottom on the following cases :-

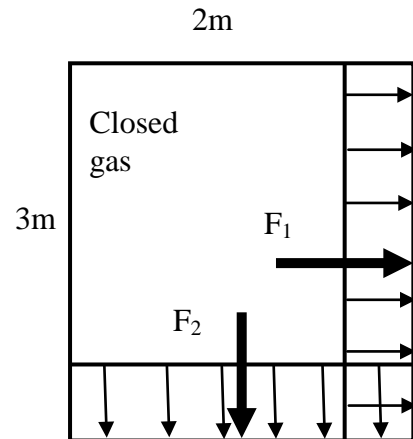
- 1 – Tank is closed containing gas of pressure 5 bar
- 2 – Tank is opened containing water height of 2.5 m



## 1 – gas at 5 bar

a) side  $F_1 = PA_1$   
 $= 5 * 10^5 * 2 * 3$   
 $= 3 * 10^6 \text{ N}$   
 $= 3 \text{ MN} \quad \perp \text{ side at center}$

b) bottom  $F_2 = PA_2$   
 $= 5 * 10^5 * 2 * 2$   
 $= 2 * 10^6 \text{ N}$   
 $= 2 \text{ MN} \quad \perp \text{ bottom at center}$



## 2 – Water with 2.5 height

a) side  $F_1 = \gamma_w h_0 * \frac{h_0}{2} * B$   
 $= 9800 * \frac{(2.5)^2}{2} * 2$   
 $= \text{N} \quad \perp \text{ side at } \frac{2.5}{3} \text{ from bottom}$

b) bottom  $F_2 = \gamma_w h_0 * A$   
 $= 9800 * 2.5 * (2*2)$   
 $= \text{N} \quad \perp \text{ bottom at center}$

